

Kathy Strom, *Mayor*  
Patricia Burda, *Vice Mayor*  
Al Lang, *Secretary*  
Vicky Taplin, *Treasurer*  
John Bickerman, *Community Liaison*

## Notice of Water Drainage Plan Submission

**To:** Residents Abutting and Confronting 7402 Ridgewood Avenue  
**Fr:** Dave Walton, Town of Chevy Chase  
**Date:** April 9, 2015  
**Re:** Water Drainage Plan Submission

The Town of Chevy Chase has received a water drainage plan and report for the construction of a new house at 7402 Ridgewood Avenue. Chapter 28 of the Town Code requires that a water drainage plan be submitted if new impervious surface created by a development activity and all other development activities within the 2 year period prior to filing an application exceeds 700 square feet. According to the application, the new house and associated improvements will result in 5,480 square feet of impervious area.

If approved, the proposed on-site drainage facilities will consist of six drywells, as shown on the attached site plan. The drywells are designed to collect water runoff from the downspouts and driveway and store it below the ground until it is absorbed into the subsoils. The system will provide a total storage volume of 823 cubic feet. According to calculations provided with the application, the total volume required to be infiltrated is 474.9 cubic feet. The calculations are under review by the Town's consulting engineer.

Residents are free to submit comments regarding the proposed water drainage plan. Please submit any comments on the proposed plan by Thursday, April 16. The Town will provide these comments to the Town engineer. The Town Attorney has advised that the engineer is not obligated to incorporate these comments into his review if the plan otherwise meets the requirements of the water drainage ordinance; however, staff has instructed the engineer to give due consideration to any comments received.

The plan and report are available for public inspection in the Town Office between 8:30 a.m. and 5:00 p.m. Monday through Friday (please call ahead if you would like to review them).

Attachment: Water Drainage Plan

Sent To: 7400, 7401, 7402, 7403, 7404, 7405 Ridgewood Avenue  
7307, 7309, 7311 Maple Avenue

CC: Town Council





*Experience you can build on.*

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**civil • surveying • land planning**

# **WATER DRAINAGE COMPUTATIONS**

The Town of  
Chevy Chase

**7402 RIDGEWOOD AVENUE  
LOT 30, BLOCK 4  
CHEVY CHASE PARK**

**PREPARED FOR:**

Chase Builders  
Attn: Carlos Fernandes  
8750 Brookville Road  
Silver Spring, MD 20910  
(301) 588-4747 Phone  
(301) 588-4757 Fax

**PREPARED BY:**

CAS Engineering  
108 West Ridgeville Boulevard, Suite 101  
Mount Airy, Maryland 21771  
(301) 607-8031 phone  
(301) 607-8045 fax  
Attn: Brent D. Allgood

**SUBMITTED TO:**

The Town of Chevy Chase  
4301 Willow Lane  
Chevy Chase, Maryland 20815  
Attn: William Bissell, P.E.  
301-654-7144

**March, 2015**



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## NARRATIVE

The subject property, 7402 Ridgewood Avenue, Chevy Chase is shown on Tax Map HN342 as Lot 30, Block 4, Chevy Chase Park. The 12,834.5 square foot lot is currently zoned R-60. The property lies along the west side of Ridgewood Avenue approximately 490-feet north of Thornapple Street. The lot currently slopes from south to north.

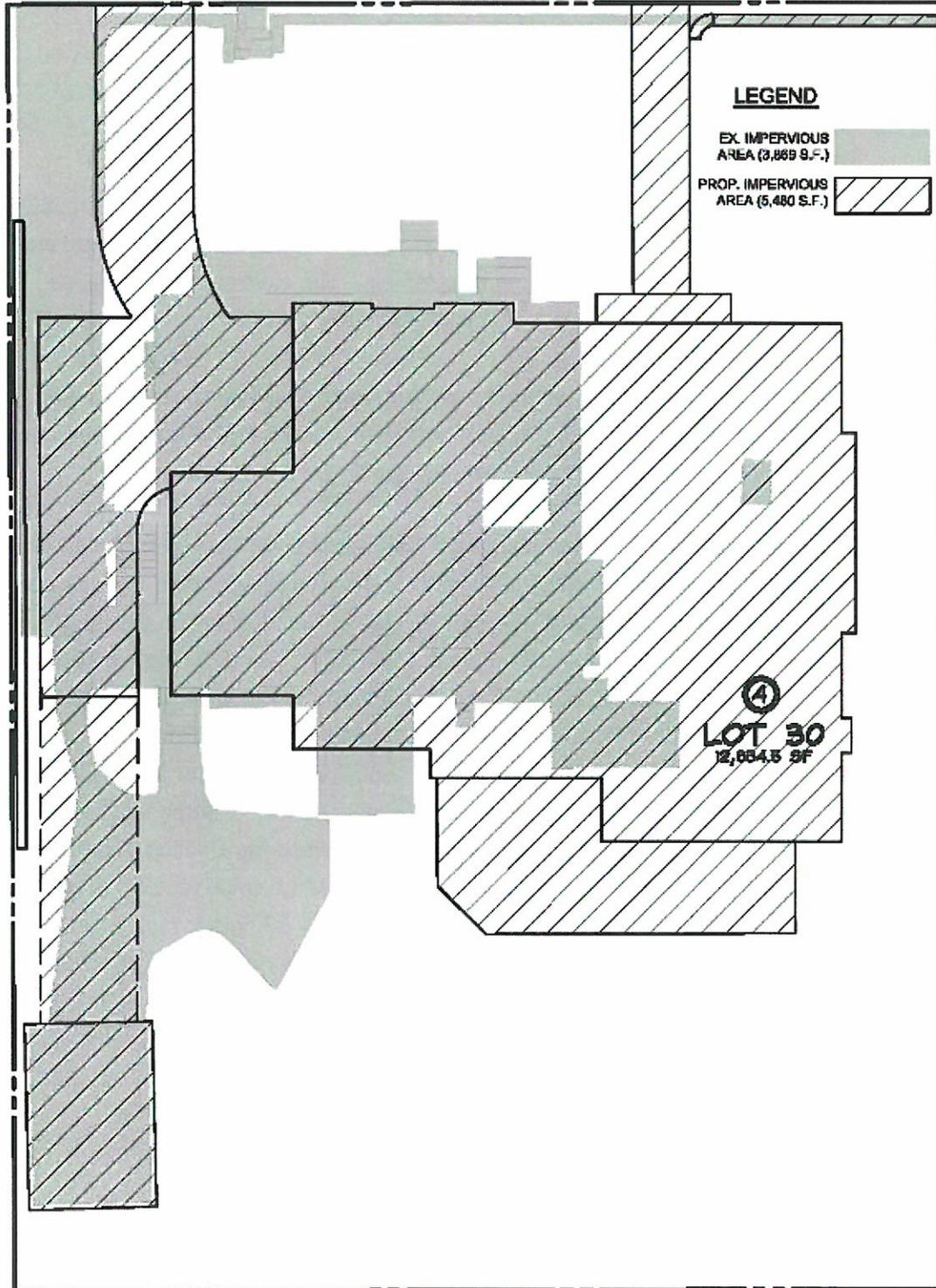
Existing improvements include a single-family detached dwelling, detached garage, retaining walls, and a gravel driveway. The existing house and portions of the existing retaining walls are to be removed. The existing detached garage is to remain. The existing improvements constitute approximately 3,869 square feet of impervious area.

Proposed improvements include a new single family detached dwelling, covered porches, patios and driveway. The proposed improvements constitute 5,480 square feet of impervious area and include portions of the existing retaining walls and garage that are to remain.

The following calculations along with the soils report (attached hereto as Appendix A) will support the proposed water drainage design. All of the proposed downspouts will be piped directly to the proposed Water Drainage Systems "A", "B", "C", "D", "E" or "F". Drainage systems "A", "B" and "F" are located in the front yard, while "C", "D" and "E" are located in the rear yard. Drainage systems "A", "B", "C" and "D" will consist of gravel drywells. Drainage systems "E" and "F" will consist of RainTank™ Triple Modules. Each system will include a pop-up emitter which will serve as the overflow release for rainfall events greater than the 3-month storm (Q3-month). These drywells will provide the necessary storage volume for the proposed 3-month storm, which is greater than the runoff difference between the existing and proposed 10-year storms. Total storage volume required by the town is 474.9 cubic feet. As designed, water drainage systems "A", "B", "C", "D", "E" and "F" will provide 823 cubic feet, thus exceeding the minimum Town requirements. Additionally, other impervious surfaces are hydraulically disconnected and cannot be treated. These Best Management practices along with the placement of topsoil on all disturbed areas, prior to seeding or sodding, will provide control for the 3-month runoff.

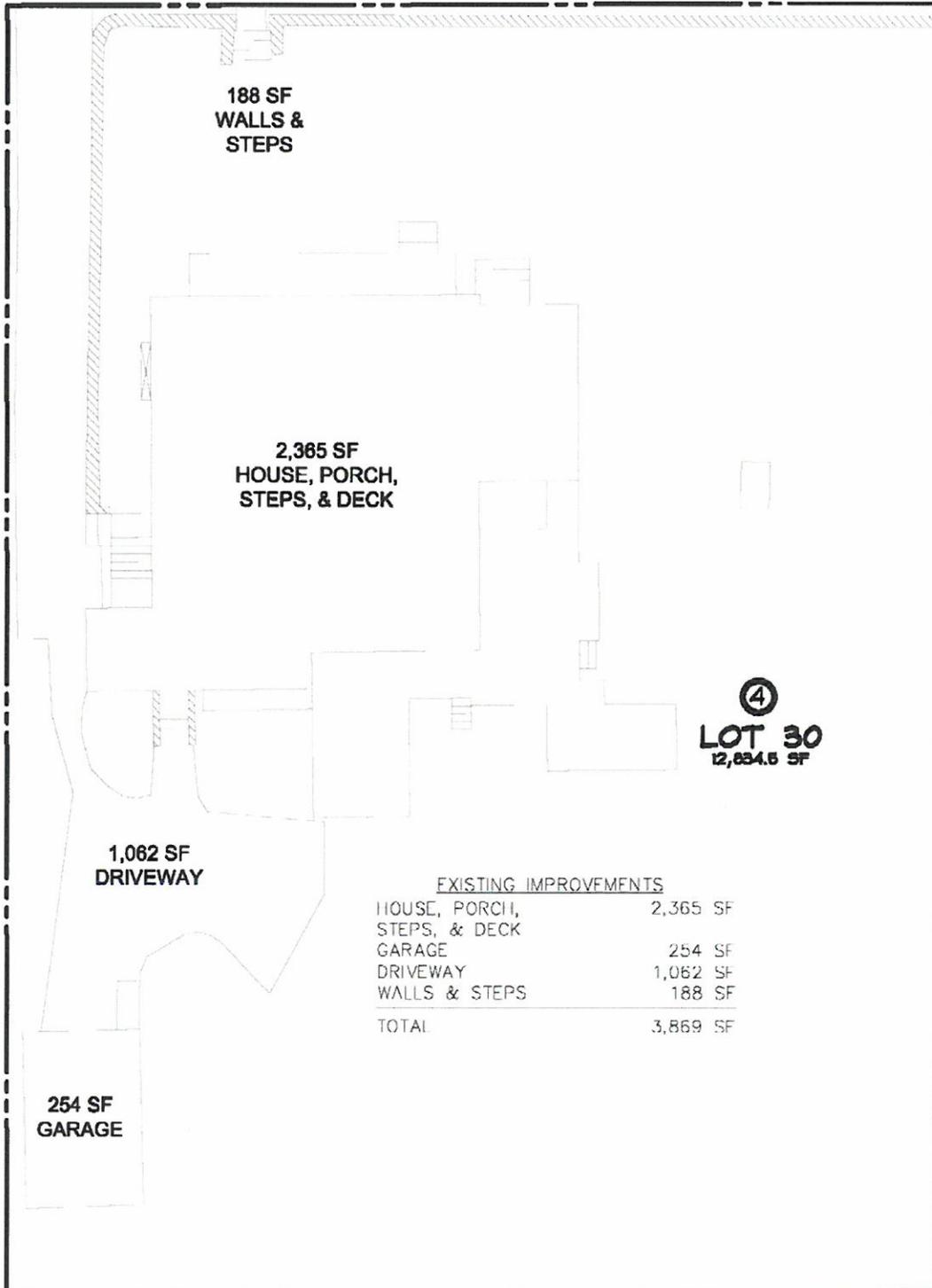
## ANALYSIS OF ON-SITE DRAINAGE AREAS

Existing vs. Proposed Conditions



## ANALYSIS OF ON-SITE DRAINAGE AREAS

### Existing Conditions





## ANALYSIS OF ON-SITE DRAINAGE AREAS

### Existing Conditions

IMPROVEMENT/ AREA	AREA (Square Feet)	CURVE NUMBER (CN)
House, Porch, Steps & Deck	2,365 SF	98
Garage	254 SF	98
Driveway	1,062 SF	98
Walls & Steps	188 SF	98
Green Space	1,611 SF *	75
<b>TOTAL</b>	<b>IMPERVIOUS AREA 3,869 SQUARE FEET</b>	

\* Green space area equates to difference between existing and proposed impervious areas

#### DETERMINE $Q_{10\text{-year}}$ , given $P = 5.1$ inches

$$Q_{10\text{-year}} = \frac{(P - 0.2S)^2}{(P + 0.8S)} \qquad S = \frac{1000}{CN} - 10 = (1000/98) - 10 = 0.20$$

$$Q_{10\text{-year}} = \frac{((5.1 - 0.2(0.20))^2}{((5.1 + 0.8(0.20))} = 4.87 \text{ INCHES}$$

$Q_{10\text{-year}} = 4.87 \text{ INCHES}$  (from impervious areas)

$$Q_{10\text{-year}} = \frac{(P - 0.2S)^2}{(P + 0.8S)} \qquad S = \frac{1000}{CN} - 10 = (1000/75) - 10 = 3.33$$

$$Q_{10\text{-year}} = \frac{((5.1 - 0.2(3.33))^2}{((5.1 + 0.8(3.33))} = 2.53 \text{ INCHES}$$

$Q_{10\text{-year}} = 2.53 \text{ INCHES}$  (from grass areas)



**DETERMINE REQUIRED RETENTION VOLUME,  $V_{10\text{-year}}$**

$$V_{10\text{-year}} = Q_{10\text{-year}} \times \text{IMPERVIOUS AREA}$$

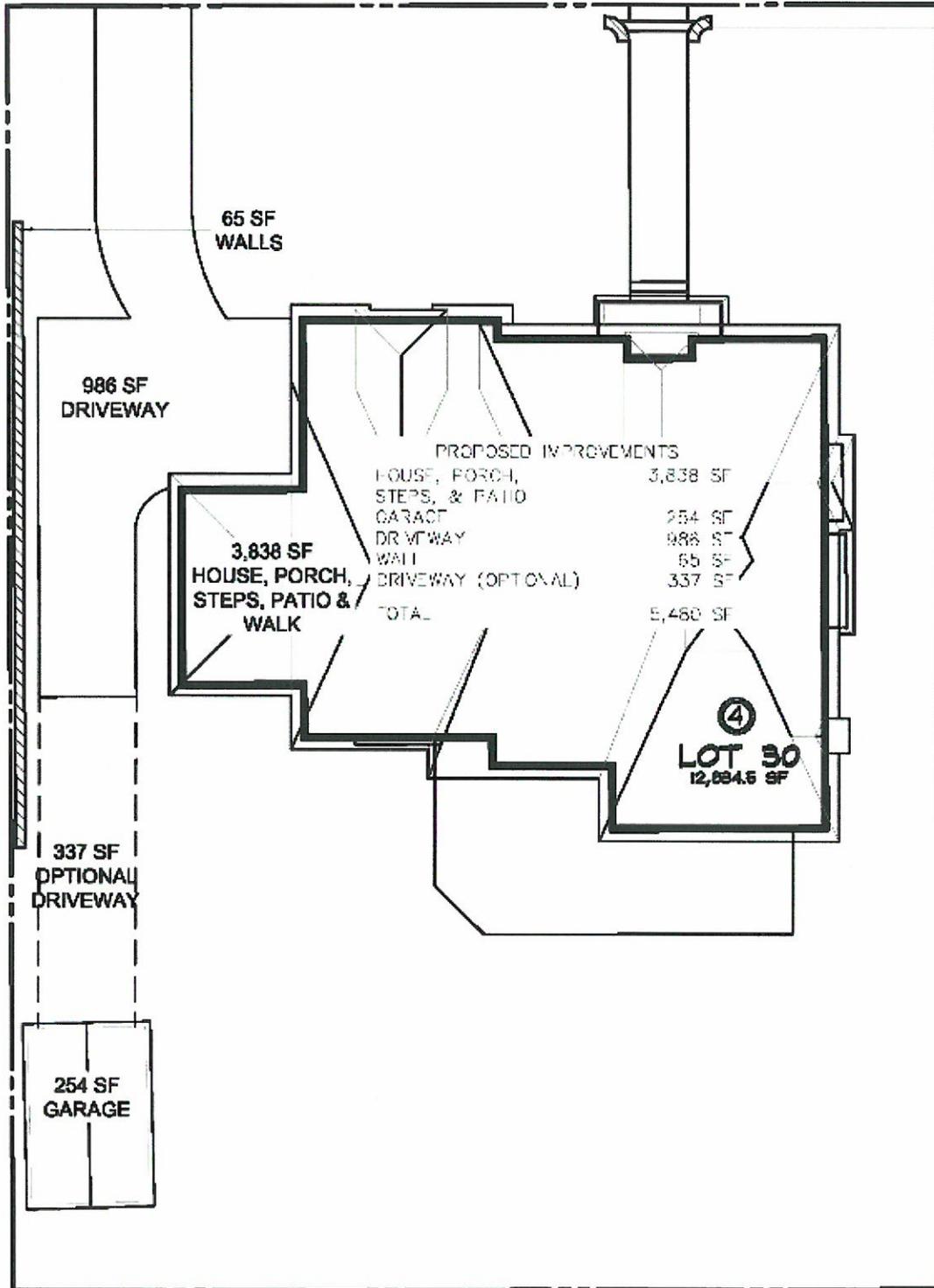
$$= 4.87 \text{ INCHES} \times 3,869 \text{ SF} \times 1 \text{ FT} / 12 \text{ INCHES} = 1,570.2 \text{ CF}$$

$$= 2.53 \text{ INCHES} \times 1,611 \text{ SF} \times 1 \text{ FT} / 12 \text{ INCHES} = 339.7 \text{ CF}$$

$$V_{10\text{-year}(\text{total})} = \mathbf{1,909.9 \text{ CUBIC FEET}}$$

**ANALYSIS OF ON-SITE DRAINAGE AREAS**

**Proposed Conditions**





## ANALYSIS OF ON-SITE DRAINAGE AREAS

### Proposed Conditions

IMPROVEMENT/ AREA	AREA (Square Feet)	CURVE NUMBER (CN)
House, Porch, Steps & Patio	3,838 SF	98
Garage	254 SF	98
Driveway	986 SF	98
Walls & Steps	65 SF	98
Driveway (optional)	337 SF	98
<b>TOTAL</b>	<b>IMPERVIOUS AREA 5,480 SQUARE FEET</b>	<b>98</b>

**DETERMINE  $Q_{3\text{-Month}}$ , given  $P = 1.25$  inches**

$$Q_{3\text{-Month}} = \frac{(P - 0.2S)^2}{(P + 0.8S)} \qquad S = \frac{1000}{CN} - 10 = (1000/98) - 10 = 0.20$$

$$Q_{3\text{-Month}} = \frac{((1.25 - 0.2(0.20))^2}{((1.25 + 0.8(0.20))} = 1.04 \text{ INCHES}$$

$$Q_{3\text{-Month}} = 1.04 \text{ INCHES}$$

**DETERMINE  $Q_{10\text{-year}}$ , given  $P = 5.1$  inches**

$$Q_{10\text{-year}} = \frac{(P - 0.2S)^2}{(P + 0.8S)} \qquad S = \frac{1000}{CN} - 10 = (1000/98) - 10 = 0.20$$

$$Q_{10\text{-year}} = \frac{((5.1 - 0.2(0.20))^2}{((5.1 + 0.8(0.20))} = 4.87 \text{ INCHES}$$

$$Q_{10\text{-year}} = 4.87 \text{ INCHES}$$



## **ANALYSIS OF ON-SITE DRAINAGE AREAS**

### **Proposed Conditions**

#### **DETERMINE REQUIRED RETENTION VOLUME, $V_{3\text{-Month}}$ & $V_{10\text{-year}}$**

$$\begin{aligned} V_{3\text{-Month}} &= Q_{3\text{-Month}} \times \text{IMPERVIOUS AREA} \\ &= 1.04 \text{ INCHES} \times 5,480 \text{ SF} \times 1 \text{ FT} / 12 \text{ INCHES} = 474.9 \text{ CF} \end{aligned}$$

$$V_{3\text{-Month}} = \mathbf{474.9 \text{ CUBIC FEET}}$$

$$\begin{aligned} V_{10\text{-year}} &= Q_{10\text{-year}} \times \text{IMPERVIOUS AREA} \\ &= 4.87 \text{ INCHES} \times 5,480 \text{ SF} \times 1 \text{ FT} / 12 \text{ INCHES} = 2,224.0 \text{ CF} \end{aligned}$$

$$V_{10\text{-year}} = \mathbf{2,224.0 \text{ CUBIC FEET}}$$

#### **DETERMINE INCREASE IN RUNOFF FOR 10-YEAR STORM**

$$V_{10\text{-year}(\text{EX})} = \mathbf{1,909.9 \text{ CUBIC FEET}}$$

$$V_{10\text{-year}(\text{PROP})} = \mathbf{2,224.0 \text{ CUBIC FEET}}$$

$$V_{10\text{-year}(\text{PROP})} - V_{10\text{-year}(\text{EX})} = 2,224.0 - 1,909.9 = 314.1 \text{ CUBIC FEET}$$

#### **DESIGN VOLUME TO RETAIN**

Retain the larger of **474.9 cubic feet ( $V_{3\text{-Month}}$ )** or **314.1 cubic feet ( $V_{10\text{-year}}$  Difference).**



## **STORM DRAINAGE FACILITY DESIGN**

### **DETERMINE VOLUME TO BE RETAINED BY GRAVEL DRYWELLS AND RAIN TANK™ TRIPLE MODULES**

**Design Storage Volume =  $V_{\text{DESIGN}} = 474.9$  CUBIC FEET**

Soil report prepared by ECS Mid-Atlantic, LLC (See Attached)

Soil Test Location "B-1" has an infiltration rate of 0.48 in/hour at 7.0-feet deep, Soil Test Location "B-2" has an infiltration rate of 0.6 in/hour at 6.4-feet deep.

Two gravel drywells and nine RankTank™ Triple Modules will be located in the front yard that corresponds to infiltration test "B-1".

#### **Drywell A**

15.3' (L) x 4.8' (W) x 4.8' (H) x 0.40 (void ratio)  
Storage Provided = **141 C.F.**

#### **Drywell B**

19.1' (L) x 4.3' (W) x 4.8' (H) x 0.40 (void ratio)  
Storage Provided = **158 C.F.**

#### **Drywell F**

9 Triple Modules x 12.28 cubic feet / module  
Storage Provided = **110 C.F.**

**Total Storage Provided by Drywells A, B & F =  $V_{\text{PROVIDED}} = 409$  C.F.**

Two gravel drywells and thirteen RankTank™ Triple Modules will be located in the rear yard that corresponds to infiltration test "B-2".

#### **Drywell C**

8.9' (L) x 8.0' (W) x 5.0' (H) x 0.40 (void ratio)  
Storage Provided = **142 C.F.**

#### **Drywell D**

16.6' (L) x 6.0' (W) x 5.0' (H) x 0.40 (void ratio)  
Storage Provided = **199 C.F.**

#### **Drywell E**

10 Triple Modules x 12.28 cubic feet / module  
Storage Provided = **122 C.F.**

**Total Storage Provided by Drywells C, D & E =  $V_{\text{PROVIDED}} = 463$  C.F.**

**Total Storage Provided by all devices =  $V_{\text{PROVIDED}} = 872$  C.F.**

*Refer to Civil Plans for additional information.*



## **STORM DRAIN PIPE DESIGN**

**Use the Rational Method to determine  $Q_{10}$  for roof drain pipe**

$$Q_{10} = C \times I_{10} \times A$$

Assume time of concentration,  $T_c = 5$  minutes

$$I_{10} = 7.07 \text{ in /hour}$$

$C = 0.90$  (roof surface, 100% impervious)

$A = 969$  square feet = 0.0222 acres (largest combined roof area conveyed to drywell)

$$Q_{10} = C \times I_{10} \times A = 0.9 \times 7.07 \times 0.0222 = 0.141 \text{ CFS}$$

$$\mathbf{Q_{10} = 0.14 \text{ CFS}}$$

**The runoff from largest roof surface generated from the 10-year storm is 0.14 CFS.**

Design roof drain pipe to convey  $Q_{10} = 0.14 \text{ CFS}$ . Assume a minimum slope of 2%.



## FlowMaster™ Computation

### CIRCULAR CHANNEL ANALYSIS & DESIGN SOLVED WITH MANNING'S EQUATION

#### OPEN CHANNEL – UNIFORM FLOW

**Worksheet Name:** 7402 Ridgewood Avenue

**Comment:** 4" Schedule 40 PVC roof drains

#### Solve for Actual Depth

##### Given Input Data:

Diameter .....	0.33 ft (4" PVC)
Slope .....	0.02 ft/ft (2% Min. Slope)
Manning's n .....	0.011
Discharge .....	0.14 cfs (see Q <sub>10</sub> calculation, page 10)

##### Computed Results:

Depth .....	0.16 ft
Velocity .....	3.53 fps
Flow Area .....	0.04 sf
Critical Slope .....	0.0075 ft/ft
Critical Depth .....	0.21 ft
Percent Full .....	47.15%
Froude Number .....	1.79
Full Capacity .....	0.31 cfs
QMAX @ .94D .....	0.33 cfs

Open Channel Flow Module, Version 3.12 (c) 1990  
Haestad Methods, Inc. \* 37 Brookside Road, Waterbury, Ct 06708

**A 4" schedule 40 PVC pipe will be sufficient to convey the 10-year runoff from the largest combined roof area.**

**Provide 4" PVC for individual downspouts.**



## **MAINTENANCE**

### **Drywell Maintenance**

Drywells will be installed with an observation well which should be monitored quarterly for the first year and annually thereafter.

Contributing runoff originates from roofed surfaces. All gutters will be equipped with gutter drain filters (or similar device) to prevent leaves from entering the drywells. Additionally, cleanouts will be provided along longer pipe runs. All upstream structures access points must be monitored for accumulation of sediment and debris on the same schedule as stated above. All accumulation of sediment and debris should be removed promptly.

### **Drywell Maintenance Schedule**

During the first year after construction of the drywells, the system(s) should be monitored on a quarterly basis as well as after significant storms. A log book shall be maintained and shall indicate: date, time, and depth of water in the observation well at 8 to 12 hour intervals for a 48-hour period. Once the performance characteristics of the system have been verified, and warranted, the monitoring schedule can be reduced to an annual basis.

Sediment build-up inside the drywell and in all upstream storm drain structures should be monitored on the same schedule as described above. Sediment deposits shall be removed on a quarterly basis from all upstream structures including gutters. Once the performance characteristics of the system have been verified, and warranted, the sediment removal schedule can be reduced to an annual basis.

Ponding, standing water, or algal growth on the top of a drywell may indicate failure due to sedimentation in the gravel media. If water ponds for more than 48 hours after a major storm or more than six inches of sediment has accumulated the gravel media should be excavated and replaced.

Repair and / or replacement of drywells to be in accordance with Town, County and State requirements applicable at the time of the inspection.

## STORM DRAINAGE COST ESTIMATE

### 7402 Ridgewood Avenue

CONSTRUCTION COST ESTIMATE FOR  
STORMWATER DRAINAGE SYSTEM

ITEM	QUANTITY	UNIT COST INSTALLED	TOTAL COST
<b><i>SWM Facility</i></b>			
GRAVEL DRYWELLS	4	\$400.00 EA	\$1,600.00
RAINTANK TRIPLE MODULES	15	\$80.00 EA	\$1,200.00
		<b>TOTAL</b>	<b>\$2,800.00</b>

## **APPENDIX - SOILS REPORT**



August 21, 2014

Mr. Jeffrey Robertson  
CAS Engineering  
108 West Ridgeville Boulevard  
Suite 101  
Mount Airy, Maryland 21771

ECS Job No.: 13-6404

Reference: Report of Subsurface Exploration and Infiltration Test Results, CAS Project No. 14-211C, 7402 Ridgewood Avenue, Chevy Chase, Maryland

Dear Mr. Robertson:

As requested, ECS Mid-Atlantic, LLC (ECS) has completed the soil boring and infiltration testing for the proposed stormwater management (SWM) facilities at the above referenced project.

### **Soil Conditions**

Subsurface conditions within the proposed stormwater management areas were evaluated with two (2) soil test borings drilled to a maximum depth of fifteen (15) feet below the existing ground surface.

Natural soils were encountered at the ground surface at the boring locations. The natural soils extended to depths of fifteen feet below existing grades, the maximum depths explored. The natural soils were identified as Sandy SILT (ML) and Silty SAND (SM) and contained varying amounts of mica and trace amounts of gravel. Based on Standard Penetration Test (SPT) results, the natural cohesionless soils encountered ranged from loose to very dense. The color of these natural materials was grayish brown to brown and the moisture content of these soils was characterized as moist.

More detailed descriptions of the soils encountered are provided on the boring log attached to this letter.

### **Groundwater Observations**

Groundwater was not encountered during drilling of the soil borings. Observations for groundwater were made during sampling and upon completion of the drilling operations at the boring locations. In auger drilling operations, water is not introduced into the boreholes, and the groundwater position can often be determined by observing water flowing into or out of the borehole. Furthermore, visual observation of the soil samples retrieved during the auger drilling exploration can often be used in evaluating the groundwater conditions.

The highest groundwater observations are normally encountered in winter and early spring. Variations in the location of the long-term water table may occur as a result of changes in precipitation, evaporation, surface water runoff, and other factors not immediately apparent at

the time of this exploration. Perched water may also be encountered at the interface of coarse and fine-grained soils.

### **Infiltration Testing**

In order to evaluate potential infiltration at this property, two in-situ infiltration tests were performed on August 14, 2014. The tests were conducted between 6.4 and 7.0 feet below existing grades. The test locations were selected and located in the field by ECS.

The in-situ infiltration testing consisted of auguring a soil probe down to the test depth and installing a solid length of five inch diameter PVC pipe. The pipe was then presoaked for 24 hours by filling the pipe with approximately two feet of water. After the initial filling of the pipe, infiltration testing was completed by monitoring the drop in the water level at 60-minute intervals for four hours. The rate of drop over the four total hours is considered the infiltration rate. The test results are as shown in the table on the following page.

<b>Test Location</b>	<b>Test Depth (ft)</b>	<b>Approximate Test Elevation (ft)</b>	<b>Soil Encountered at Test Depth</b>	<b>Field Infiltration Rate (in/hr)</b>
B-1	7.0	315	Loose Sandy SILT (ML)	0.48
B-2	6.4	314.6	Medium Dense to Loose Silty SAND (SM)	0.6

The results reported above are based on field measurements. We recommend that the design rate be calculated as 2/3 of the field rate to account for siltation over time.

This report has been prepared to aid in the evaluation of this site and to assist the design team with the design of the on-site stormwater management facilities. The report scope is limited to this specific project and the location described. The project description represents our current understanding of the significant aspects of the proposed improvements relevant to the geotechnical considerations.

7402 Ridgewood Avenue SWM  
ECS Job No. 13-6404  
August 21, 2014  
Page 3

We appreciate the opportunity to have provided geotechnical engineering services on this project. Should you have questions regarding our findings or need additional consultations, please do not hesitate to contact our office.

Respectfully,

**ECS MID-ATLANTIC, LLC**

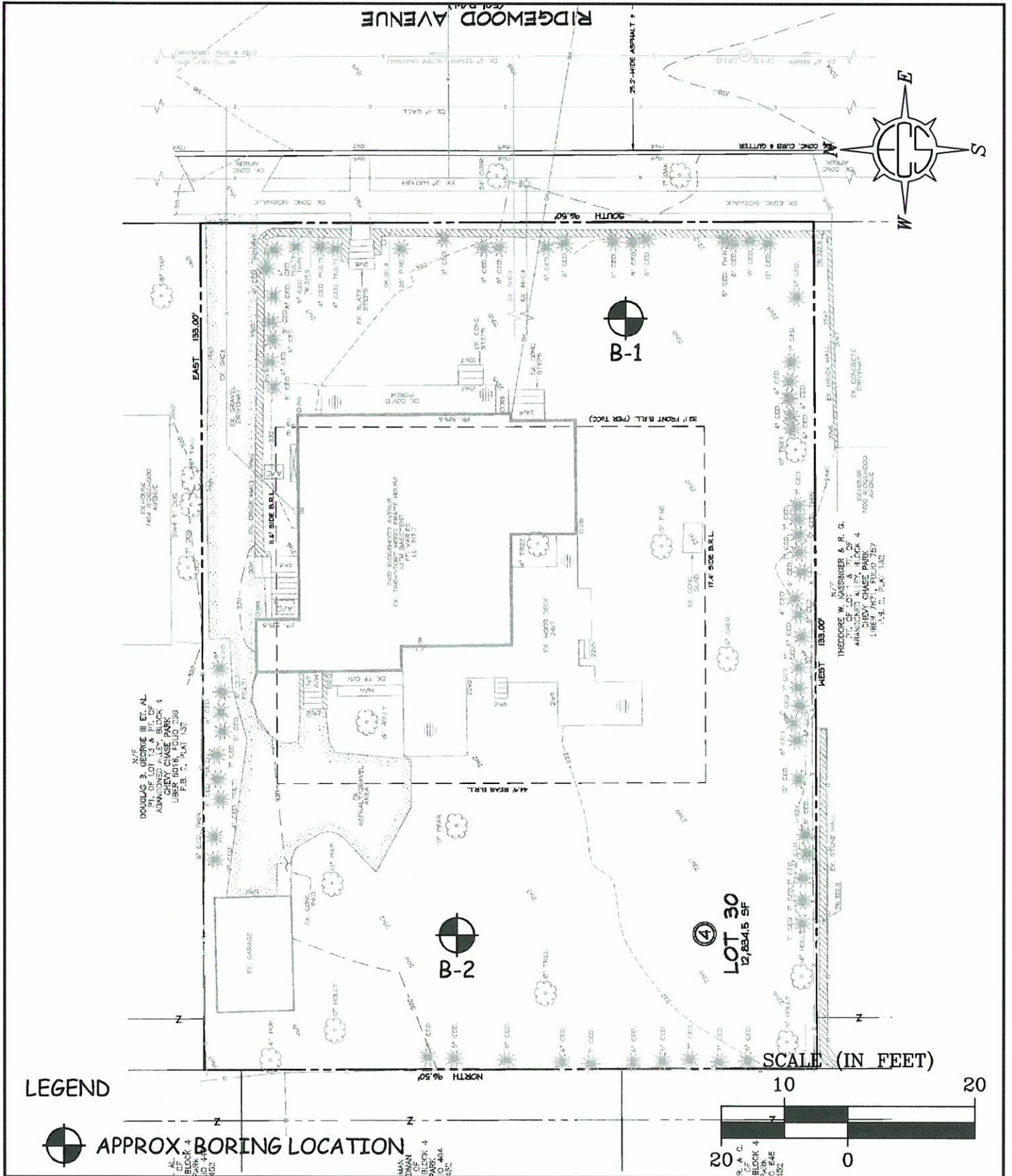


Brian A. Meley, P.G.  
Geotechnical Project Manager

Jeffrey A. McGregor, P.E.  
Principal Engineer

Enclosure: Boring Location Diagram (1 page)  
Boring Logs (2 pages)

I:\Department 3 Geotechnical\GEOTECHNICAL\PROJECTS\6400's\13-6404 7402 Ridgewood Avenue SWM\13-6404 7402 Ridgewood Ave SWM Report.doc



**BORING LOCATION  
DIAGRAM**

**CAS ENGINEERING**



**7402 RIDGEWOOD  
AVENUE SWM**

**CHEVY CHASE, MARYLAND**

ENGINEER	BAM	SCALE	1" = 20'
DRAFTSMAN	AMH	PROJECT NO.	13-6404
REVISIONS		SHEET	1 OF 1
		DATE	08-14-14

CLIENT <b>CAS Engineering</b>	JOB # <b>6404</b>	BORING # <b>B-1</b>	SHEET <b>1 OF 1</b>	
PROJECT NAME <b>7402 Ridgewood Avenue SWM</b>	ARCHITECT-ENGINEER			

SITE LOCATION  
**7402 Ridgewood Avenue, Chevy Chase,**

NORTHING \_\_\_\_\_ EASTING \_\_\_\_\_ STATION \_\_\_\_\_

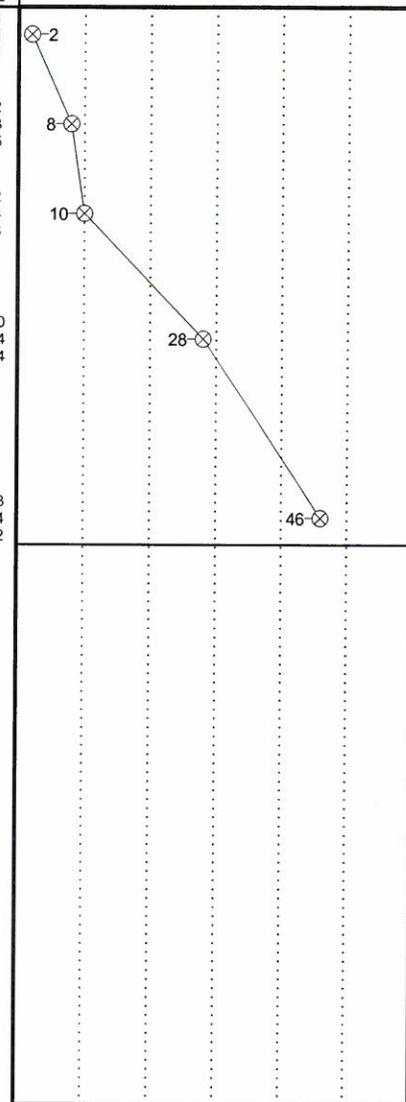
DEPTH (FT)	SAMPLE NO.	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	ENGLISH UNITS	WATER LEVELS ELEVATION (FT)	BLOWS/ft	ROCK QUALITY DESIGNATION & RECOVERY		
									RQD% - - -	REC% - - -	
0					Topsoil Depth [18"]				○	○	△
	S-1	SS	18	6	(ML) SANDY SILT, Contains Significant Mica, Brown, Moist, Loose				○	○	△
	S-2	SS	18	15					○	○	△
5	S-3	SS	18	15					○	○	△
10	S-4	SS	18	18	(ML) SANDY SILT, Contains Significant Mica, Grayish Brown, Moist, Medium Dense				○	○	△
15	S-5	SS	18	18	(SM) SILTY SAND, Contains Mica, Grayish Brown, Moist, Dense				○	○	△
					END OF BORING @ 15.00'						

○ CALIBRATED PENETROMETER TONS/FT<sup>2</sup>

ROCK QUALITY DESIGNATION & RECOVERY  
RQD% - - - REC% - - -

PLASTIC LIMIT%      WATER CONTENT%      LIQUID LIMIT%

⊗ STANDARD PENETRATION BLOWS/FT



THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL.

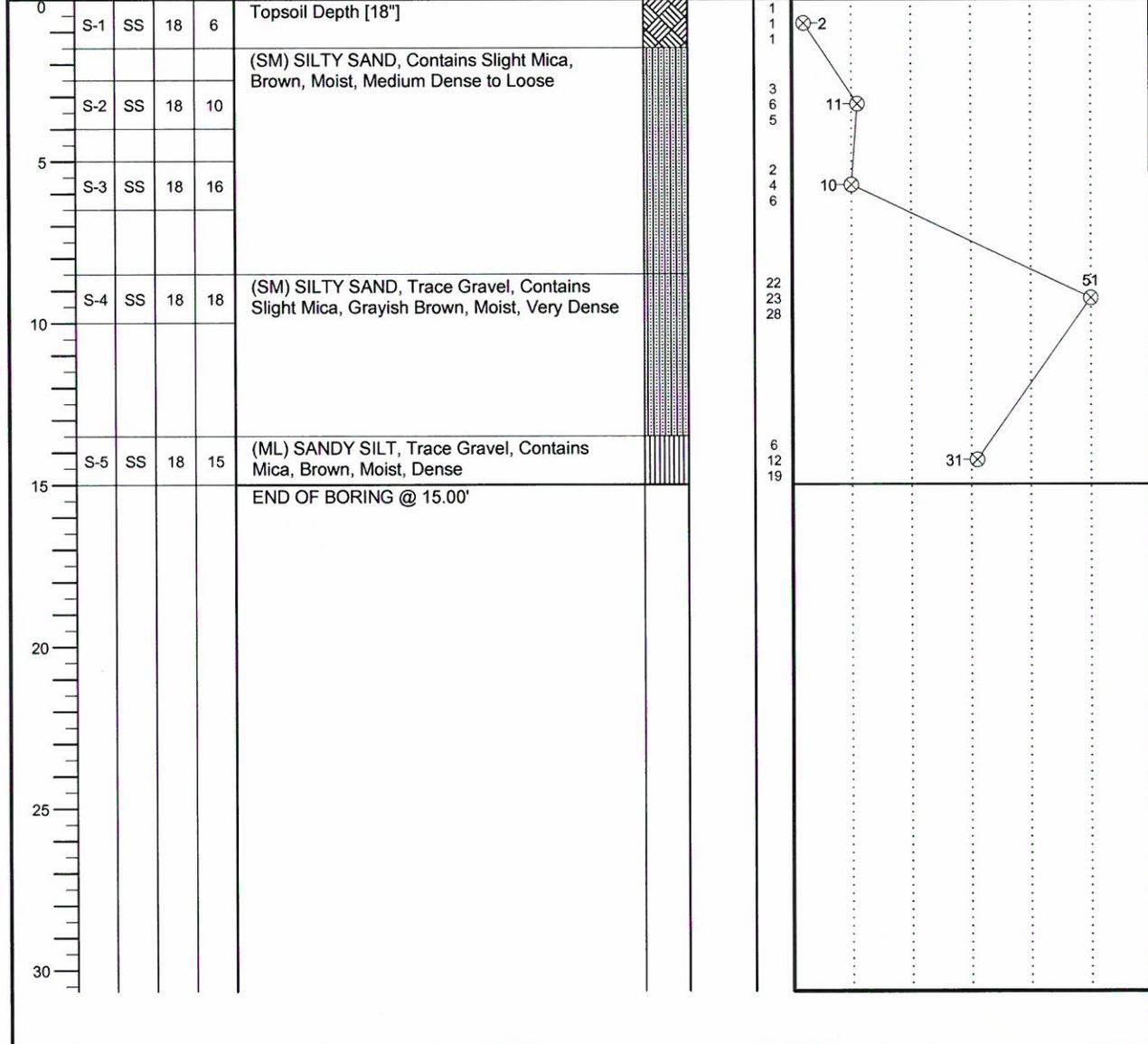
WL	WS <input type="checkbox"/> WD <input type="checkbox"/>	BORING STARTED	08/13/14	
WL(BCR)	WL(ACR)	BORING COMPLETED	08/13/14	CAVE IN DEPTH @ 10.00'
WL		RIG 550 ATV	FOREMAN Zack	DRILLING METHOD HSA

CLIENT <b>CAS Engineering</b>	JOB # <b>6404</b>	BORING # <b>B-2</b>	SHEET <b>1 OF 1</b>	
PROJECT NAME <b>7402 Ridgewood Avenue SWM</b>	ARCHITECT-ENGINEER			

SITE LOCATION  
**7402 Ridgewood Avenue, Chevy Chase,**

NORTHING	EASTING	STATION
----------	---------	---------

DEPTH (FT)	SAMPLE NO.	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	ENGLISH UNITS	WATER LEVELS	ELEVATION (FT)	BLOWS/6"
------------	------------	-------------	-------------------	---------------	-------------------------	---------------	--------------	----------------	----------



○ CALIBRATED PENETROMETER TONS/FT<sup>2</sup>

ROCK QUALITY DESIGNATION & RECOVERY  
RQD% - - - REC% - - -

PLASTIC LIMIT%      WATER CONTENT%      LIQUID LIMIT%

⊗ STANDARD PENETRATION BLOWS/FT

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL.

WL	WS <input type="checkbox"/>	WD <input type="checkbox"/>	BORING STARTED	08/07/14	
WL(BCR)	WL(ACR)		BORING COMPLETED	08/07/14	CAVE IN DEPTH
WL			RIG 550 ATV	FOREMAN Zack	DRILLING METHOD HSA

PT. OF LOT 29 & 30  
PT. OF  
LEY, BLOCK 4  
ASE PARK  
FOLIO 440  
LAT 452

ABANDONED ALLEY, BLOCK 4  
CHEVY CHASE PARK  
LIBER 5016, FOLIO 280  
P.B. 2, PLAT 132

PROP. 15" INDIAN  
INLINE DRAIN  
(OR APPROVED EQUIVALENT),  
SEE DETAIL ON SHEET 2

IW: 319.8  
BW(N): 319.5  
BW(S): 319.8  
HEIGHT: 0.3'

IW: 319.9  
BW(N): 320.8  
BW(S): 320.8  
HEIGHT: 0.9'

TW: 319.4  
BW(N): 319.2  
BW(S): 319.4  
HEIGHT: 0.2'

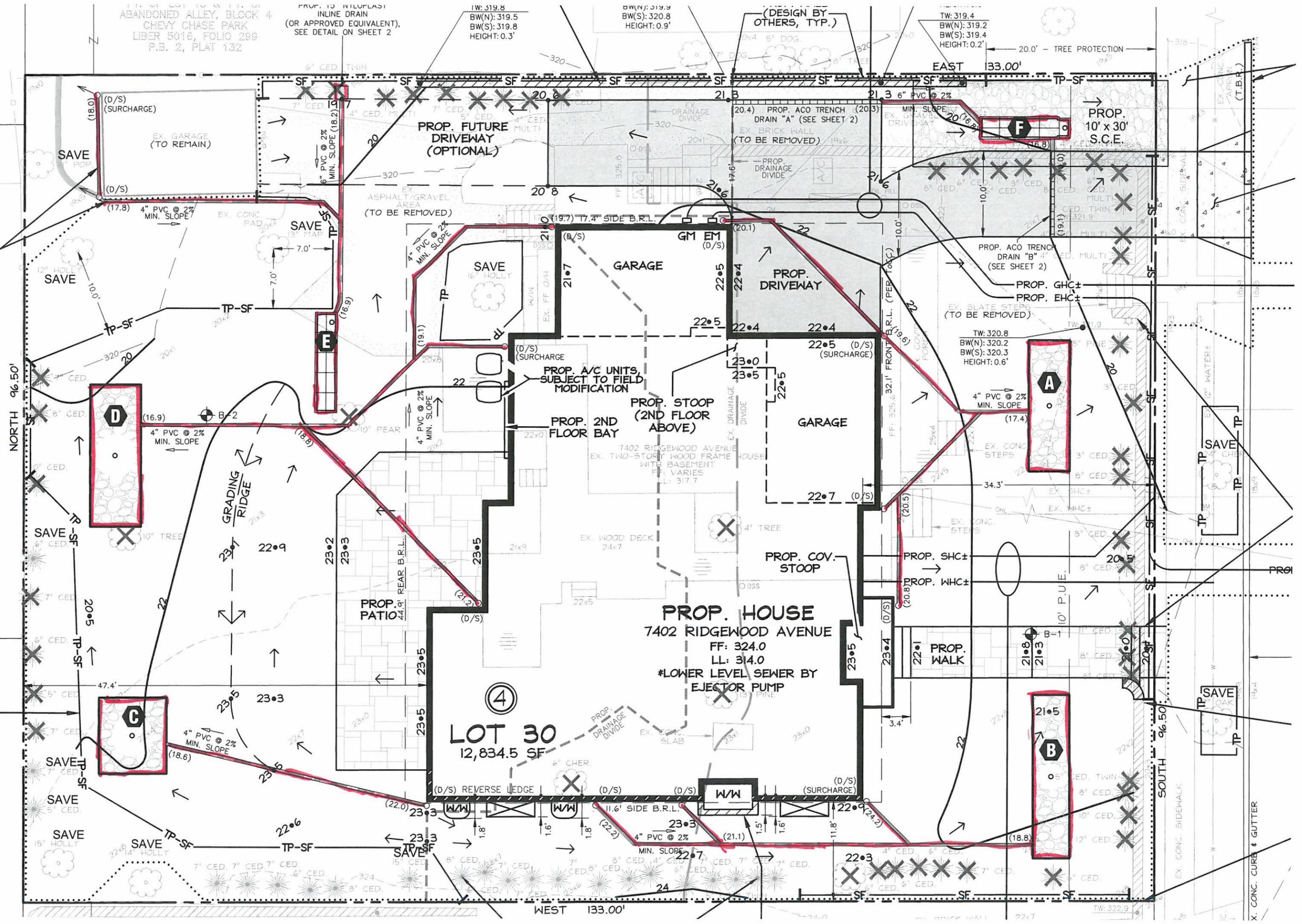
EAST 133.00'

TO BE LOCATED  
CENT TO EX.  
BE EXCAVATED  
IE TREE  
ALL OTHER PVC  
D OUTSIDE OF  
AREA.

FRIEDMAN &  
FRIEDMAN  
PT. OF  
LEY, BLOCK 4  
ASE PARK  
FOLIO 404  
LAT 452

ION AREA  
GE

ALL JR. & C. W.  
PT. OF  
LEY, BLOCK 4  
ASE PARK  
FOLIO 645  
LAT 452



LOT 30  
12,834.5 SF

PROP. HOUSE  
7402 RIDGEWOOD AVENUE  
FF: 324.0  
LL: 314.0  
\*LOWER LEVEL SEWER BY  
EJECTOR PUMP

PROP. FUTURE  
DRIVEWAY  
(OPTIONAL)

PROP. STOOB  
(2ND FLOOR  
ABOVE)

PROP. A/C UNITS,  
SUBJECT TO FIELD  
MODIFICATION

PROP. 2ND  
FLOOR BAY

PROP. COV.  
STOOB

PROP.  
10' x 30'  
S.C.E.

PROP. ACO TRENCH  
DRAIN "B" (SEE SHEET 2)

PROP. GHC±  
PROP. EHC±

EX. SLATE STEPS  
(TO BE REMOVED)

SAVE  
TP

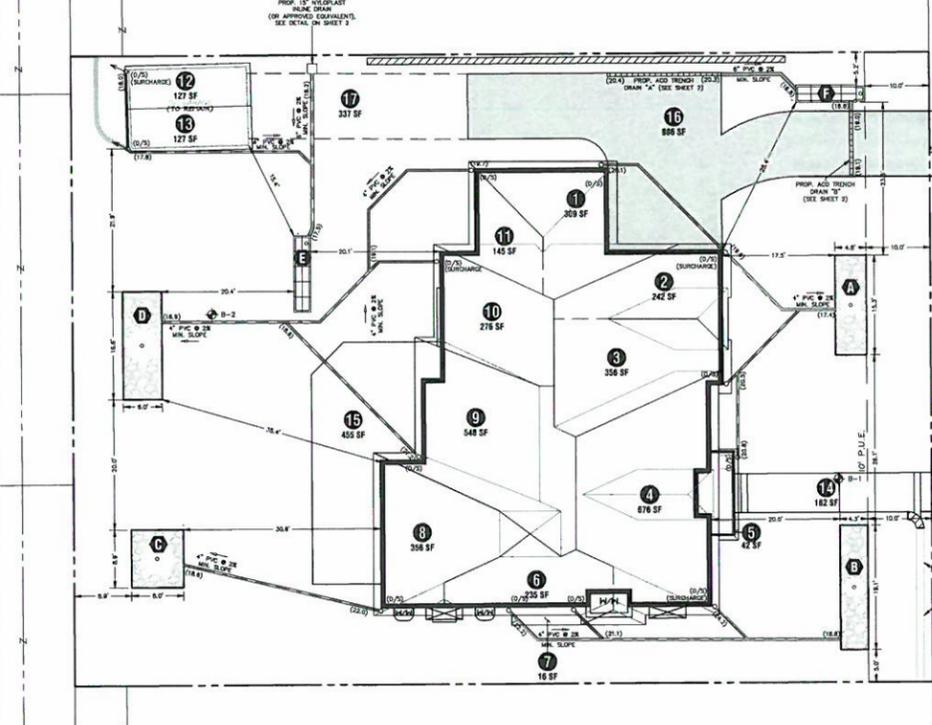
EX. CONC. CURB & GUTTER

EX. CONC. SIDEWALK



**IMPERVIOUS SURFACE AND ROOF AREA DIAGRAM**

SCALE: 1" = 10'



**ESD COMPUTATIONS - 7402 RIDGEWOOD AVENUE**

TOTAL LOT AREA FOR P DETERMINATION	PROPOSED LOT IMPERVIOUS AREA FOR P DETERMINATION	LOT IMPERVIOUS AREA PERCENTAGE FOR P DETERMINATION	P <sub>r</sub> - RAINFALL TARGET APPLY IMPERVIOUS COVER PERCENTAGE TO TABLE 3.3 - SOIL GROUP B
12,835 S.F.	5,435 S.F.	42.3%	1.8 IN.
LOFT IS 40,000 SF, USE TOTAL L.O.D. ON SITE AND IN RW AREA TO DETERMINE TARGET ESD.			
DETERMINE ESD REQUIRED FOR PROPOSED IMPERVIOUS AREAS		TARGET ESD (P <sub>r</sub> x I <sub>u</sub> ) / A <sub>i</sub> BASED ON UNIT OF DISTURBANCE	
TOTAL SITE ESD VOLUME REQUIRED: 170.9 C.F.			

DRYWELL STRUCTURE	IMPERVIOUS AREA NO.	IMPERVIOUS AREA (SQ. FT.)	MIN. ESD (IN. FT.)	NO. OF DRYWELLS (L x W x D) (FT.)	SURFACE AREA (SQ. FT.)	PROVIDED DRYWELL VOLUME (L x W x D) (C.F.)	% MAXIMUM VOLUME CHECK (L x W x D) (C.F.)	VOLUME PROVIDED VIA DRYWELLS
A ROOF	1	309	1.8	15 (LENGTH) x 8 (WIDTH)	735 F.	345 C.F.	195 C.F.	341 C.F.
	2	242	1.8	12 (LENGTH) x 8 (WIDTH)	480 F.	216 C.F.	126 C.F.	216 C.F.
	3	356	1.8	18 (LENGTH) x 8 (WIDTH)	756 F.	353 C.F.	198 C.F.	353 C.F.
	4	42	1.8	6 (LENGTH) x 8 (WIDTH)	144 F.	68 C.F.	38 C.F.	68 C.F.
	5	42	1.8	6 (LENGTH) x 8 (WIDTH)	144 F.	68 C.F.	38 C.F.	68 C.F.
TOTAL	949	676	1.8	57 (LENGTH) x 8 (WIDTH)	1155 F.	558 C.F.	317 C.F.	558 C.F.
B ROOF	6	235	1.8	12 (LENGTH) x 8 (WIDTH)	480 F.	216 C.F.	126 C.F.	216 C.F.
	7	16	1.8	6 (LENGTH) x 8 (WIDTH)	144 F.	68 C.F.	38 C.F.	68 C.F.
	8	356	1.8	18 (LENGTH) x 8 (WIDTH)	756 F.	353 C.F.	198 C.F.	353 C.F.
	9	42	1.8	6 (LENGTH) x 8 (WIDTH)	144 F.	68 C.F.	38 C.F.	68 C.F.
	10	42	1.8	6 (LENGTH) x 8 (WIDTH)	144 F.	68 C.F.	38 C.F.	68 C.F.
TOTAL	927	676	1.8	57 (LENGTH) x 8 (WIDTH)	1155 F.	558 C.F.	317 C.F.	558 C.F.
C ROOF	11	356	1.8	18 (LENGTH) x 8 (WIDTH)	756 F.	353 C.F.	198 C.F.	353 C.F.
	12	42	1.8	6 (LENGTH) x 8 (WIDTH)	144 F.	68 C.F.	38 C.F.	68 C.F.
	13	42	1.8	6 (LENGTH) x 8 (WIDTH)	144 F.	68 C.F.	38 C.F.	68 C.F.
	14	42	1.8	6 (LENGTH) x 8 (WIDTH)	144 F.	68 C.F.	38 C.F.	68 C.F.
	15	42	1.8	6 (LENGTH) x 8 (WIDTH)	144 F.	68 C.F.	38 C.F.	68 C.F.
TOTAL	580	676	1.8	30 (LENGTH) x 8 (WIDTH)	2520 F.	1224 C.F.	652 C.F.	1224 C.F.
D ROOF	16	276	1.8	14 (LENGTH) x 8 (WIDTH)	504 F.	238 C.F.	133 C.F.	238 C.F.
	17	145	1.8	7 (LENGTH) x 8 (WIDTH)	252 F.	119 C.F.	66 C.F.	119 C.F.
	18	42	1.8	6 (LENGTH) x 8 (WIDTH)	144 F.	68 C.F.	38 C.F.	68 C.F.
	19	42	1.8	6 (LENGTH) x 8 (WIDTH)	144 F.	68 C.F.	38 C.F.	68 C.F.
	20	42	1.8	6 (LENGTH) x 8 (WIDTH)	144 F.	68 C.F.	38 C.F.	68 C.F.
TOTAL	547	676	1.8	27 (LENGTH) x 8 (WIDTH)	1008 F.	483 C.F.	272 C.F.	483 C.F.

ROOFTOP DISCONNECTS	IMPERVIOUS AREA NO.	IMPERVIOUS AREA (SQ. FT.)	MIN. ESD (IN. FT.)	ROOFTOP DISCONNECT LENGTH (FT.)	P PROVIDED BY DISCONNECT (C.F.)	NO. PROVIDED BY ROOFTOP DISCONNECT (IN.)	Q PROVIDED BY DISCONNECT (C.F.)	VOLUME PROVIDED VIA DISCONNECTS
NOT TREATED	21	127	1.8	15 FEET	0.2 W	1	28 C.F.	28 C.F.
	22	14	1.8	6 FEET	0.2 W	1	28 C.F.	28 C.F.
	23	455	1.8	15 FEET	0.2 W	1	28 C.F.	28 C.F.
	24	18	1.8	6 FEET	0.2 W	1	28 C.F.	28 C.F.
	25	337	1.8	15 FEET	0.2 W	1	28 C.F.	28 C.F.
TOTAL	2,087	676	1.8	60 FEET	0.2 W	5	140 C.F.	140 C.F.

TOTAL WITH IMPERVIOUS AREA	ESD PROVIDED BY DRYWELLS	ESD PROVIDED BY DISCONNECTS	ESD PROVIDED BY DISCONNECTS (IN.)	Q PROVIDED BY DISCONNECT (C.F.)	VOLUME PROVIDED VIA DISCONNECTS
5,435 S.F.	558 C.F.	140 C.F.	0.2 W	140 C.F.	200 C.F.
TOTAL ESD PROVIDED	698 C.F.	140 C.F.	0.2 W	140 C.F.	200 C.F.
IS ESD ADEQUATE	643.32 C.F. $\approx$	170.9 C.F.	ESD TO THE M.E.P.	200 C.F.	ESD TO THE M.E.P.
IS P ADEQUATE	1.23 IN. $\approx$	1.80 IN.	ESD TO THE M.E.P.		ESD TO THE M.E.P.

\* ESD volume has been provided to the Maximum Extent Practicable (M.E.P.) through the use of disconnects and gravel drywells. The use of other Environmental Site Design (E.S.D.) practices, specifically landscape infiltration, was explored but is not feasible due to the limited feasible locations in regards to grading and the proximity of proposed devices to the subject building, property lines, utilities and retaining walls. Additionally, permeable pavers are not feasible due to slopes greater than 6% and the proximity to the subject building, retaining walls, etc. For the reasons noted above we have not been able to provide a P<sub>r</sub> of 1.8". We hereby request a stormwater management quantity waiver. All appropriate fees will be paid at the time of sediment control permit issuance.

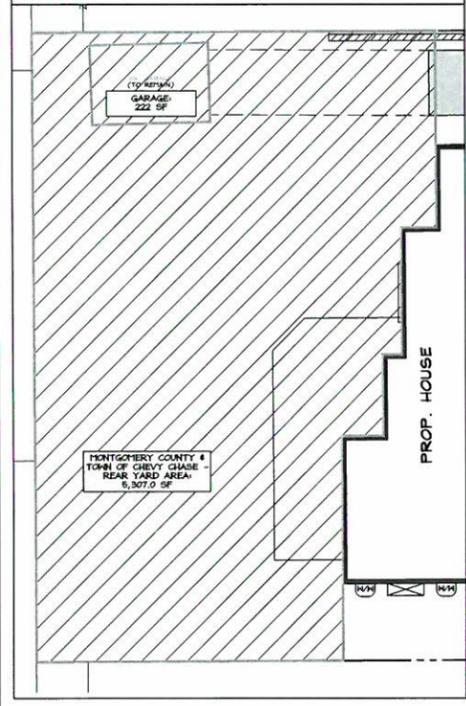
**GRAVEL DRYWELL SCHEDULE - 7402 RIDGEWOOD AVENUE**

DRYWELL STRUCTURE (SCHEDULE)	FINISHED GRADE (LOW SIDE)	FINISHED GRADE (HIGH SIDE)	ELEVATION AT TOP OF GRAVEL (E <sub>1</sub> )	PIPE INVERT IN FROM DOWNPOUTS	TOTAL DEPTH OF GRAVEL (E <sub>2</sub> )	ELEVATION AT BOTTOM OF GRAVEL (E <sub>3</sub> )	TOTAL DEPTH OF SAND (E <sub>4</sub> )	ELEVATION AT BOTTOM OF SAND (E <sub>5</sub> )	TOTAL DEPTH OF SAND (E <sub>6</sub> )	TOTAL DEPTH OF SAND (E <sub>7</sub> )	TOTAL DEPTH OF SAND (E <sub>8</sub> )	RECOMMENDED OVERFLOW
A	320.4	320.8	318.4	317.4	3.8 IN.	314.6	1.0 IN.	313.6	4.8 IN.	7.2 IN.	7.2 IN.	POP UP EMITTER AT DRYWELL CLEANOUT AND A SURCHARGE PIPE AT THE LOWEST DOWNPOUT PER DRYWELL
B	321.8	322.7	315.8	314.8	3.8 IN.	311.0	1.0 IN.	310.0	4.8 IN.	7.7 IN.	7.7 IN.	
C	321.6	322.3	315.6	314.6	4.0 IN.	311.6	1.0 IN.	310.6	5.0 IN.	7.6 IN.	7.6 IN.	
D	322.9	323.3	314.9	313.9	4.0 IN.	311.9	1.0 IN.	310.9	5.0 IN.	7.4 IN.	7.4 IN.	

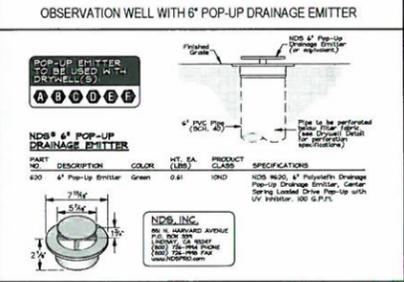
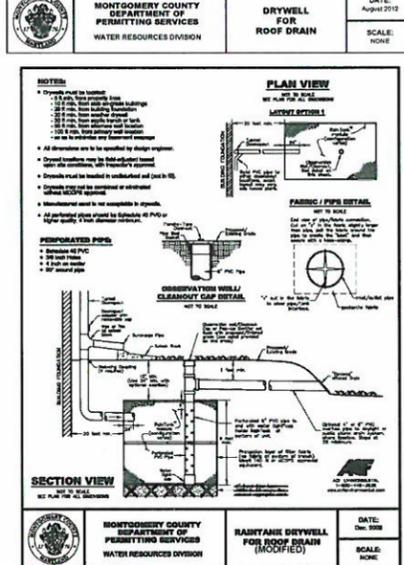
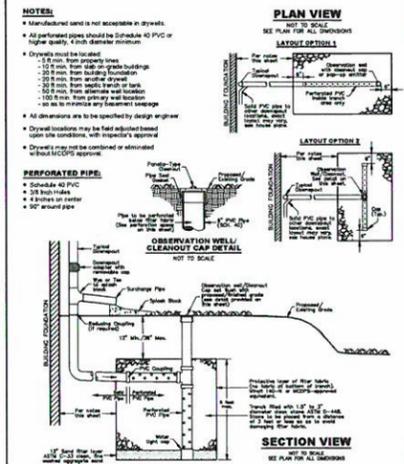
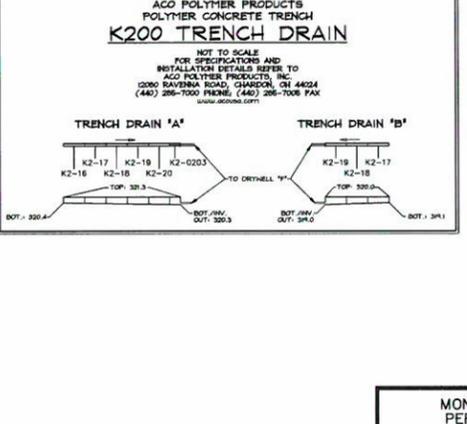
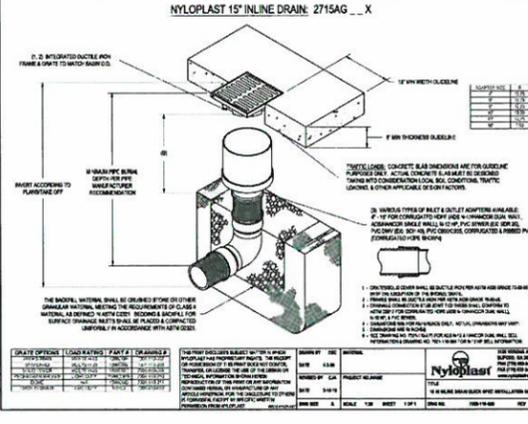
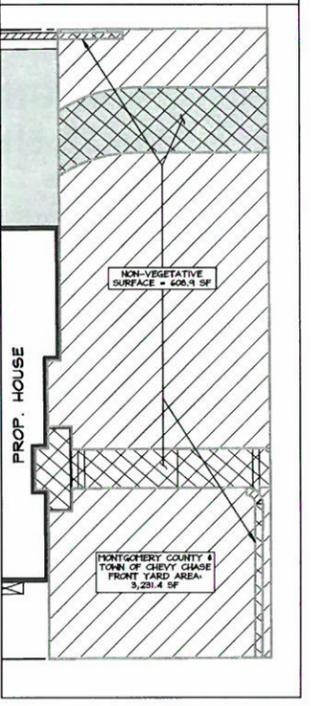
**RAINTANK™ DRYWELL SCHEDULE - 7402 RIDGEWOOD AVENUE**

DRYWELL STRUCTURE (SCHEDULE)	FINISHED GRADE (LOW SIDE)	FINISHED GRADE (HIGH SIDE)	ELEVATION AT TOP OF RAIN-TANK (E <sub>1</sub> )	PIPE INVERT IN FROM DOWNPOUTS	ELEVATION AT BOTTOM OF RAIN-TANK (E <sub>2</sub> )	TOTAL DEPTH OF RAIN-TANK (E <sub>3</sub> )	# OF RAIN-TANKS (E <sub>4</sub> )	RECOMMENDED OVERFLOW
E	320.4	321.7	317.9	316.9	313.7	4.2 IN.	10	POP UP EMITTER AT DRYWELL CLEANOUTS AND A SURCHARGE PIPE AT THE LOWEST DOWNPOUTS PER DRYWELL
F	322.3	322.2	317.5	316.5	313.3	4.2 IN.	9	

**REAR YARD LOT COVERAGE (N.T.S.)**



**FRONT YARD NON-VEGETATIVE COVERAGE (N.T.S.)**



**SAND SPECIFICATIONS**

WASHED ASTM C-33 FINE AGGREGATE CONCRETE SAND IS UTILIZED FOR STORMWATER MANAGEMENT APPLICATIONS IN MONTGOMERY COUNTY. IN ADDITION TO THE ASTM C-33 SPECIFICATION, SAND MUST MEET ALL OF THE FOLLOWING CONDITIONS:

- Sand must meet gradation requirements for ASTM C-33 Fine Aggregate Concrete Sand. ASTM D-155-8 gradation is also acceptable.
- Sand must be silica-based, no limestone based products may be used. If the material is white or gray in color, it is probably not acceptable.
- Sand must be clean, friable, unweathered sand deposits may not be used. Likewise, sand that has become contaminated by improper storage or installation practices will be rejected.
- Manufactured sand or stone dust is not acceptable under any circumstances.

**STORM DRAIN NOTES**

- ALL STORM DRAIN PIPE TO BE SCHEDULE 40 PVC OR HIGHER QUALITY (I.E. CAST IRON).
- DOWNPOUT LEADERS ORIGINATING DIRECTLY FROM DOWNPOUTS TO BE 4\"/>

**\*\*FOR TOWN OF CHEVY CHASE, SHEET 2 OF 2\*\***

**APPLICANT:** CHASE BUILDERS  
6700 BROOKVILLE ROAD  
SILVER SPRING, MARYLAND 20910  
(301) 666-4747 PHONE  
(301) 666-4757 FAX  
ATTN: CARLOS FERNANDES  
carlos@chasebuilders.com

**ARCHITECT:** STUDIO Z DESIGN CONCEPTS  
8200 HOOVER AVE, SUITE 160  
BETHESDA, MARYLAND 20814  
(301) 981-8811 PHONE  
(301) 981-8833 FAX  
ATTN: PAUL DAVEY  
paul@stzdesign.com

7402 RIDGEWOOD AVENUE  
LOT 30, BLOCK 4  
CHEVY CHASE PARK  
- TOWN OF CHEVY CHASE -  
BUILDING PERMIT SITE PLAN,  
STORMWATER MANAGEMENT PLAN,  
AND SEDIMENT CONTROL PLAN

**MONTGOMERY COUNTY DEPARTMENT OF PERMITTING SERVICES APPROVED FOR:**

NOTE: MDCPS APPROVAL DOES NOT NEGATE THE NEED FOR A MDCPS ACCESS PERMIT.

STORMWATER MANAGEMENT:	SEDIMENT CONTROL TECHNICAL REQUIREMENTS:	ADMINISTRATIVE REQUIREMENTS:
REVIEWED _____ DATE _____	REVIEWED _____ DATE _____	REVIEWED _____ DATE _____
APPROVED _____ DATE _____	APPROVED _____ DATE _____	SEDMENT CONTROL PERMIT NUMBER: 270295
APPROVED _____ DATE _____	APPROVED _____ DATE _____	MDCPS APPROVAL OF THIS PLAN WILL EXPIRE TWO YEARS FROM THE DATE OF APPROVAL IF THE PROJECT HAS NOT STARTED.

DATE: 03/31/2015  
CURT A. SCHREFFLER, PE

DATE: 3/2015  
PROJECT: 14-21C  
SCALE: 1" = 10'

108 West Ridgeway Blvd., Suite 101  
Mount Airy, Maryland 21771  
301-407-5033 office  
301-407-5033 cell  
www.casengineering.com  
info@casengineering.com

CHEVY CHASE PARK  
LOT 30, BLOCK 4  
BUILDING PERMIT SITE PLAN,  
STORMWATER MANAGEMENT PLAN,  
AND SEDIMENT CONTROL PLAN

COS ENGINEERING  
Experience you can build on.

2 OF 3

